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# Retrofitting timber house foundations on sloping sites

Houses built on sloping sites have performed poorly in recent earthquakes. BRANZ experiments have shown how to retrofit pre-1960s timber house foundations to significantly improve seismic performance.

**MANY NEW ZEALAND** houses have been built – and continue to be built – with timber foundations on sloping sites.

Observations from the Christchurch earthquake of 2011 and the subsequent BRANZ House Condition Survey showed that houses on these sites tended to perform poorly. There is a need to improve the design of these to ensure greater resilience.

More resilient houses will mean a greater opportunity for residents to remain in their houses following a major earthquake – an important consideration in the aftermath of such an event.

# Differing stiffness causes problems

The problem arises because the uphill foundation of these types of houses usually consists of a squat foundation wall, whereas the downhill foundation is a tall structure of piles or poles. The uphill edge, therefore, has a stiff foundation under lateral loading, and the downhill edge has a flexible foundation.

Under earthquake action across the slope, there is potential for large displacements on the tall flexible foundation structure on the downslope edge of the building, causing it to twist about the squatter, stiffer uphill foundation (Figure 1).

The deflections from the twisting action adds to the lateral displacement, potentially resulting in a progressive rotational failure. This difference in stiffness and the potential torsional problem is not addressed in NZS 3604:2011 *Timber-framed buildings*, where only the foundation's strength is considered, nor in a specific engineering design if only strength is considered.



## Four different foundations tested

To shed light on this issue, BRANZ partnered with Victoria University to conduct an experimental study jointly funded by the Building Research Levy and EQC. Four tests were carried out on representative foundations on a sloping site on a farm close to BRANZ, just north of Wellington.

Scala penetrometer testing established that the soil bearing properties closely approximated the lower limit of 'good ground', as defined by NZS 3604:2011.

#### Pre-1960s and NZS 3604:2011 foundations

Each test specimen consisted of a timber-framed plywood-clad floor of approximately 2.4 m across the slope by 4.8 m up and down the slope. All four floors had a concrete foundation wall supporting the upper edge and piles at the lower edge. See Figure 2 for the floors under construction:

- Floor 1 standard NZS 3604:2011 construction using braced piles at the lower edge of the floor.
- Floor 2 standard NZS 3604:2011 construction using ordinary piles.

- Floor 3 typical pre-1960s construction using concrete piles and jack studs with cut between braces (see Figure 3a).
- Floor 4 typical pre-1960s construction using concrete piles and jack studs and clad with horizontal timber weatherboards (see Figure 3b).

#### Weight added and shaking begins

Dead weight was added to the floor of each specimen to replicate the true mass of the superstructure associated with the supporting foundation.



Figure 2

 Test foundations being built – NZS 3604:2011 floors 1 and 2 on the right and pre-1960 floors 3 and 4 on left.





Figure 4

Pre-1960s downhill subfloor wall construction after strengthening.

## Piles rocked noticeably

It was clear that all pile foundations rocked quite noticeably under the lateral loading, creating gaps between soil and concrete at the end of the tests. This was also observed in the foundations of some Christchurch buildings after the earthquakes.

# Retrofitting improved performance

During the shaking, deflections of the pre-1960s floors increased, almost to the point of imminent collapse in one test. To address this, the pre-1960s foundations were retrofitted to improve their performance:

- Both floors 3 and 4 had infill concrete foundation walls cast between the existing piles (see Figure 4a and 4b).
- Floor 3 had all frame joints strengthened with the installation of galvanised brackets at all stud to plate joints (Figure 4a).
- Floor 4 had sheets of 9 mm plywood nailed to the inside face of the jack stud wall (see Figure 4b).
  These retrofit measures greatly improved the floor performance.





Under the initial shaking, the pre-1960s floor structures proved to be quite flexible. However, after retrofitting, they stiffened up considerably, as anticipated by the measurements taken in the preliminary tests. Figure 5 shows the relative stiffness and the increase in relative stiffness in both pre-1960s foundations.

# Results can be applied to houses

The NZS 3604:2011 floors performed overall as expected, although the rocking of the foundations was a little greater than anticipated.

A substantial increase in performance to the pre-1960s foundations was achieved by the retrofit solutions.

The infill foundation walls were very effective at stabilising the isolated pile foundations, although these could be difficult to construct on sites with limited underfloor access.

Nominal reinforcement was used in the wall, together with grouted in starters to all the piles. In the interests of buildability, starters could be reduced to end piles only as the centre piles are book-ended by foundation walls on each side.

The ply sheeting was very straightforward to install, as were the extra connectors on the jack stud braces. All solutions are likely to be inexpensive for most residential foundations.